LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA & LOADS BELOW OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.

GENERAL REQUIREMENTS

b. SEATING STRUCTURE LIVE LOADS:

- THE DESIGN AND CONSTRUCTION OF THIS PROJECT IS GOVERNED BY THE "FLORIDA BUILDING CODE (FBC)", SIXTH EDITION, HEREAFTER REFERRED TO AS THE FBC, AS ADOPTED AND MODIFIED BY THE AUTHORITY HAVING JURISDICTION (AHJ).
- WHERE OTHER STANDARDS ARE NOTED IN THE DRAWINGS, USE THE LATEST EDITION OF THE STANDARD UNLESS A SPECIFIC DATE IS INDICATED. REFERENCE TO A SPECIFIC SECTION IN A CODE DOES NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE ENTIRE
- THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH).
- ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE
- ALL BUILDING SITES SHALL BE GRADED TO PROVIDE DRAINAGE UNDER ALL PORTIONS OF THE BUILDING AND AROUND THE BUILDING
- PERIMETER TO ALLOW DRAINAGE AWAY FROM THE STRUCTURE. SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH,
- SHOP DRAWINGS SHALL BE REVIEWED BY CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW WILL BE RETURNED UNCHECKED.
- CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY CLOUDED AND NOTED. ARCHITECT/ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL
- DISCREPANCIES, OMISSIONS, OR INCONSISTENCIES WITH APPLICABLE CODE REQUIREMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER IN WRITING BEFORE SUBMITTING A BID OR PROCEEDING WITH THE WORK.
- 10. THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO EARTHWORK, FOUNDATIONS, SHORING, AND EXCAVATION. ANY UTILITY INFORMATION SHOWN ON THE DRAWINGS AND DETAILS IS APPROXIMATE AND NOT NECESSARILY
- 11. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER IN WRITING BEFORE PROCEEDING WITH THE WORK.
- 12. THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. DO NOT LOAD STRUCTURES, NEW OR EXISTING, WITH WEIGHT THAT WILL ENDANGER STRUCTURE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

1. I CERTIFY THAT THE PLANS AND SPECIFICATIONS COMPLY WITH THE STRUCTURAL PORTION OF THE FLORIDA BUILDING CODE SIXTH

DEMOLITION NOTES

- 1. PLANNING AND EXECUTING OF DEMOLITION SHALL BE DONE IN ACCORDANCE WITH APPLICABLE ANSI-A10 STANDARDS.
- ALL WORK SHALL BE DONE IN ACCORDANCE WITH ALL LOCAL CODE AND ORDINANCES.
- CONTRACTOR SHALL TAKE APPROPRIATE MEASUREMENTS IF LEAD-BASE PAINT OR ASBESTOS IS FOUND OR SUSPECTED TO BE ON THE
- CONTRACTOR SHALL TAKE MEASUREMENTS TO MINIMIZE THE AMOUNT OF DUST CREATED, THE POTENTIAL RELEASE OF MOLD INTO THE ATMOSPHERE, AND THE AMOUNT OF VIBRATION PRODUCED THAT MIGHT AFFECT SURROUNDING STRUCTURES OR THE OPERATION OF NEARBY EQUIPMENT
- VERIFY LOCATION OF ALL NEW WALLS PRIOR TO REMOVING ANY EXISTING WALLS.
- ERECT AND MAINTAIN TEMPORARY BRACING, SHORING, LIGHTS, BARRICADES, SIGNS AND OTHER MEASURES AS NECESSARY TO PROTECT THE PUBLIC, THE WORKERS AND ADJOINING PROPERTY FROM DAMAGE FROM DEMOLITION WORK.
- OPEN DEPRESSIONS AND EXCAVATIONS PART OF THIS WORK SHALL BE BARRICADED AND POSTED WHEN ACCESSIBLE THROUGH ADJACENT PROPERTY OR PUBLIC ACCESS.
- PROTECT ALL EXISTING AREAS AND UTILITIES THAT ARE NOT TO BE DISTURBED.
- 9. VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS PRIOR TO STARTING WORK.
- 10. IN CASE OF ANY DISCREPANCIES BETWEEN PLANS AND EXISTING FIELD CONDITIONS. NOTIFY ENGINEER.
- 11. ALL MATERIALS TO BE REMOVED FROM SITE AND DISPOSED OF IN AN APPROVED MANNER. BURYING OF TRASH AND DEBRIS ON THE SITE IS NOT PERMITTED.
- 12. REMOVE AND REPLACE ALL ROTTEN, DECAYED OR DAMAGED WOOD FROM EXISTING STRUCTURE. 13. DURING DEMOLISHING DISCOVERY, ANY MATERIAL THAT IS DAMAGED FROM AGE, WATER, WIND ETC., INCLUDING BUT NOT LIMITED TO ALL
- CORRODED OR RUSTED METAL, WATER DAMAGED MATERIALS, ROOFING MATERIALS TO BE REPLACED FOR SAME IN KIND.
- 14. ANY DAMAGE TO EXISTING FIRE RATED PARTITIONS, SHAFTS OR FIRE PROTECTION SURFACES TO BE REPAIRED TO COMPLY WITH REQUIRED FIRE PROTECTION CODES.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE", LATEST EDITION.
- 2. STRUCTURAL STEEL GRADES AND MIN. STRESS SHALL BE AS FOLLOWS:

a. W SHAPE ASTM A992 FY = 50 KSI, FU = 65 KSI b. M, S, C MC, L SHAPES ASTM A36 FY = 36 KSI, FU = 58 KSIc. PLATES AND BARS (CARBON) ASTM A36 FY = 36 KSI, FU = 58 KSI d. ANCHOR BOLTS (HIGH STRENGTH) ASTM A325 FU = 120KSI e. TWIST-OFF TENSION CONTROL BOLTS ASTM F1852 f. ANCHOR RODS ASTM F1554 FY = 36 KSI, FU = 58 KSI ASTM A36 FY = 36 KSI, FU = 58 KSIg. THREADED RODS

ASTM A563 h. NUTS ASTM F436 i. HARDENED STEEL WASHERS j. DIRECT-TENSION INDICATING WASHERS ASTM F959

- BOLTS SHALL BE HIGH STRENGTH, BEARING TYPE IN SNUG TIGHT CONDITION, UNLESS NOTED OTHERWISE. TIGHTEN BY AN AISC APPROVED
- METHOD. WHERE FULLY PRETENSIONED OR SLIP CRITICAL BOLTS ARE REQUIRED, TIGHTENING SHALL BE ACHIEVED USING EITHER TWIST-OFF TENSION CONTROL BOLTS OR DIRECT TENSION INDICATING WASHERS. PROPER SURFACE PREPARATION IS REQUIRED FOR SLIP CRITICAL BOLTS, INCLUDING OMISSION OF PRIMER OR FIRE PROOFING, AS APPROPRIATE.
- WELDING OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH A.W.S. D1.1 WITH E70XX ELECTRODES. FILLET WELDS SHALL BE MIN.
- 3/16" UNLESS NOTED OTHERWISE. 6. ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR CONDITIONS SHALL BE HOT DIPPED GALVANIZED PER ASTM A123 AND TOUCHED UP AFTER
- SHOP PAINT ALL NON-GALVANIZED STEEL W/ A LEAD CHROMATE FREE ALKYD, SSPC PAINT 20 TYPE 1 OR 2 MIN. DRY FILM THICKNESS 3 MILS. CLEAN STEEL TO THE REQUIREMENTS OF SSPC-SP6 PRIOR TO PRIMING.
- 8. DO NOT PAINT STEEL SURFACES IN CONTACT WITH CONCRETE OR THAT ARE TO RECEIVE FIREPROOFING.

WELDING; ALL FASTENERS AND HARDWARE SHALL BE HOT DIPPED GALVANIZED PER ASTM A153.

- 9. ALL OTHER STEEL EXPOSED TO WEATHER SHALL BE PROTECTED WITH EPOXY PRIMER MARINE TYPE, UNLESS NOTED OTHERWISE.
- 10. GROUT UNDER THE BEARING PLATES (OR JACKETS) SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 5,000PSI IN 28 DAYS.

CHEMICAL (ADHESIVE) ANCHORS

AND MADE AVAILABLE TO THE EOR AS REQUESTED.

- SHALL BE TWO PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS HILTI HIT HY200, HILTI RE500 SD, POWERS PURE 100+, OR SIMPSON SET ADHESIVE SYSTEM, OR ENGINEER APPROVED SUBSTITUTION.
- EPOXY TYPES AND BRANDS VARY IN THEIR BOND STRENGTH AND SUITABILITY OF USE, DEPENDING ON TYPE OF LOADING, ANCHOR SPACING, ETC. WHEN A PARTICULAR TYPE OF EPOXY IS SPECIFIED IN THESE DRAWINGS, A UNIQUE CALCULATION HAS BEEN MADE BASED ON THE PROPERTIES OF THAT SPECIFIC TYPE OF EPOXY FOR THE SPECIFIC CONDITION SHOWN IN THE DETAIL. SUBSTITUTION OF EPOXY TYPE IS NOT ALLOWED WHERE DETAIL SPECIFIES ONLY ONE TYPE OF EPOXY, WITHOUT PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD. NOT ALL EPOXY BRANDS OR TYPES WILL BE ALLOWED AS SUBSTITUTES. ICC-ES REPORTS FOR PROPOSED ANCHOR SUBSTITUTIONS MUST BE SUBMITTED TO EOR FOR FINAL REVIEW. EOR MAY REQUIRE ENGINEERED CALCULATIONS FOR REVIEW AND **APPROVAL**
- 3. SUBSTITUTIONS OF EPOXIES IN ONE CONDITION SHALL NOT BE CONSTRUED AS APPROVAL TO MAKE SIMILAR SUBSTITUTION OF EPOXIES IN OTHER DIFFERING CONDITIONS. EACH SUBSTITUTION MUST RECEIVE PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD.
- 4. INSTALL ANCHORS IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT SPECIFIED ON DRAWINGS.
- ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI (ACI 318-11 D.9.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.
- THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS FOR ALL PRODUCTS TO BE USED PRIOR TO COMMENCEMENT OF WORK ONLY TRAINED INSTALLERS SHALL PERFORM POST INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE
- THE CONTRACTOR IS RESPONSIBLE TO ENSURE THAT ALL HOLE CLEAN-OUT REQUIREMENTS ARE FULLY COMPLETED BY THE INSTALLERS PRIOR TO INJECTING EPOXY INTO HOLES IN ACCORDANCE WITH THE MANUFACTURER'S MPII.
- NO LOAD SHALL BE APPLIED TO THE EPOXY ANCHORS UNTIL THE EPOXY HAS FULLY CURED AND HAS ACHIEVED IT'S SPECIFIED STRENGTH. CURE TIME SHALL BE PER MANUFACTURER'S PUBLISHED VALUES FOR SPECIFIC PRODUCT BEING USED.
- IF DETAIL SHOWS EPOXY ANCHORS IN SLOTTED HOLES, IT IS IMPERATIVE THAT ANY EXCESS EPOXY IS CLEANED UP FROM AROUND THE
- ANCHOR ROD, SO THAT IT DOES NOT INTERFERE WITH ADJUSTABILITY OF ANCHOR ROD IN SLOTTED HOLE. 10. ADHESIVE ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC 193
- FOR CRACKED, UNCRACKED AND SEISMIC CONCRETE RECOGNITION. ADHESIVE ANCHORS IN MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR AC106.
- 12. EXISTING REINFORCING BARS IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE CUT UNLESS APPROVED BY THE EOR 13. ADHESIVE ANCHORS IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE INSTALLED UNTIL THE CONCRETE AND/OR MASONRY HAS BEEN CURED FOR AT LEAST 21-DAYS.
- APPLICABLE BUILDING CODE AND THE CURRENT ICC-ES REPORT (IBC2012 TABLE 1705.3 NOTE B). 15. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING

14. PROVIDE SPECIAL INSPECTION FOR ALL MECHANICAL POST INSTALLED ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS OF THE

OFFICIAL (ACI 318-11 D.9.2.4). **MECHANICAL ANCHORS**

- SHALL BE EITHER HEAVY DUTY CONCRETE SCREW ANCHOR (SUCH AS POWERS WEDGE BOLT +, SIMPSON TITEN HD, OR HILTI HUS-H) OR WEDGE TYPE EXPANSION ANCHOR (SUCH AS POWERS POWER-STUD=SD1, SIMPSON WEDGE-ALL, OR HILTI KWIK BOLT 3).
- TYPE OF ANCHOR SHALL BE AS SPECIFIED ON THE DRAWINGS, WHILE BRAND AND MODEL OF ANCHOR MAY BE SELECTED FROM THE ABOVE LISTED ANCHORS. SUBSTITUTION OF ANCHORS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVED IN WRITING BY THE ENGINEER OF RECORD PRIOR TO USE. ICC-ES REPORTS FOR PROPOSED ANCHOR SUBSTITUTES MUST BE SUBMITTED TO EOR FOR REVIEW. EOR MAY REQUEST ENGINEERED CALCULATIONS FOR REVIEW AND APPROVAL.
- IN SOME CASES OF CRITICAL LOADING OR GEOMETRIC CONDITIONS, ONLY SPECIFIC ANCHORS WILL BE ALLOWED, AS NOTED ON THE DRAWINGS. IN THESE CASES, THE SPECIFIED BRAND AND MODEL OF ANCHOR MUST BE USED.
- INSTALL ANCHORS IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT SPECIFIED ON DRAWINGS.
- THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS FOR ALL PRODUCTS TO BE USED PRIOR TO COMMENCEMENT OF WORK ONLY TRAINED INSTALLERS SHALL PERFORM POST INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE AND MADE AVAILABLE TO THE EOR AS REQUESTED.
- MINIMUM EMBEDMENT DEPTH OF 1/4" TAPCONS OR POWERS TAPPER + INSTALLED IN CONCRETE SHALL BE 1.25", U.N.O. AND INSTALLED INTO MASONRY SHALL BE 1.5". SELECT ANCHOR LENGTH AS REQUIRED TO ACHIEVE THE SPECIFIED MINIMUM EMBEDMENT DEPTH.
- TAPCON SCREWS, OR POWERS TAPPER +, MAY BE REPLACED W/ 0.157" SHANK DIAMETER PAF ANCHORS (HILTI X-U, POWERS CSI, OR APPROVED EQUAL) ON A 1:1 SUBSTITUTION BASIS. MINIMUM EMBEDMENT DEPTH SHALL BE 1.25" WHEN INSTALLED INTO CONCRETE OR GROUTED MASONRY. FOLLOW MANUFACTURER'S INSTALLATION RECOMMENDATIONS, MINIMUM EDGE DISTANCES, AND PLACEMENT
- LIMITATIONS (RELATIVE TO MORTAR JOINTS IN MASONRY). MECHANICAL ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC 193 FOR CRACKED, UNCRACKED AND SEISMIC CONCRETE RECOGNITION.
- MECHANICAL ANCHORS IN MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR AC106.
- POWER ACTUATED FASTENERS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC0ES AC70. 9. EXISTING REINFORCING BARS IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE CUT UNLESS APPROVED BY THE EOR
- ANCHORS SHALL NOT BE INSTALLED IN CONCRETE AND/OR MASONRY CONSTRUCTION UNTIL THE CONCRETE AND/OR MASONRY HAS BEEN CURED FOR AT LEAST 21-DAYS.
- 11. PROVIDE SPECIAL INSPECTION FOR ALL MECHANICAL POST INSTALLED ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE AND THE CURRENT ICC-ES REPORT (IBC2012 TABLE 1705.3 NOTE B

ABBREVIATIONS - ANCHOR BOLT - ABOVE FINISHED FLOOR - INCH/INCHES - AUTHORITY HAVING JURISDICTION - INTERIOR - ALTERNATE - MASONRY APPROX - APPROXIMATELY - MAXIMUM ARCH - ARCHITECT MFR - MANUFACTURER BOTT - BOTTOM - MINIMUM - BEARING MISCELLANEOUS - COASTAL CONST. CONTROL LINE - MILES PER HOUR CCCL MPH - COLD FORMED STEEL CFS - NFW - NOT TO SCALE - CONCRETE MASONRY UNIT COI- COLUMN - ON CENTER CONC - CONCRETE - POUNDS PER SQUARE FOOT CONT CONTINUOUS - PRESSURE TREATED - REVISION/REVISED - DRILLED AND EPOXY - DOUBLE SPECS - SPECIFICATIONS DBL - DIAMETER - SCHEDULE SCHED - DIMENSION SIM. - SIMII AR - SOUTHERN YELLOW PINE SYP - DOWN - EACH - TYPICAL ELEV - ELEVATION/ELEVATOR UNO - UNLESS NOTED OTHERWISE ENGR ENGINEER - VERTICAL ΕW - EACH WAY - VERIFY IN FIELD - FXISTING - WITH - EXPANSION W/O - WITHOUT FXP -WEATHER RESISTANT BARRIER **FXTFRIOR** - FLORIDA BUILDING CODE WWF - WELDED WIRE FABRIC - WELDED WIRE MESH FINISH FLOOR WWM - STEEL REINFORCING BAR (REBAR) #5 (5/8") - FOUNDATION FND - FEET/FOOT - FOOTING

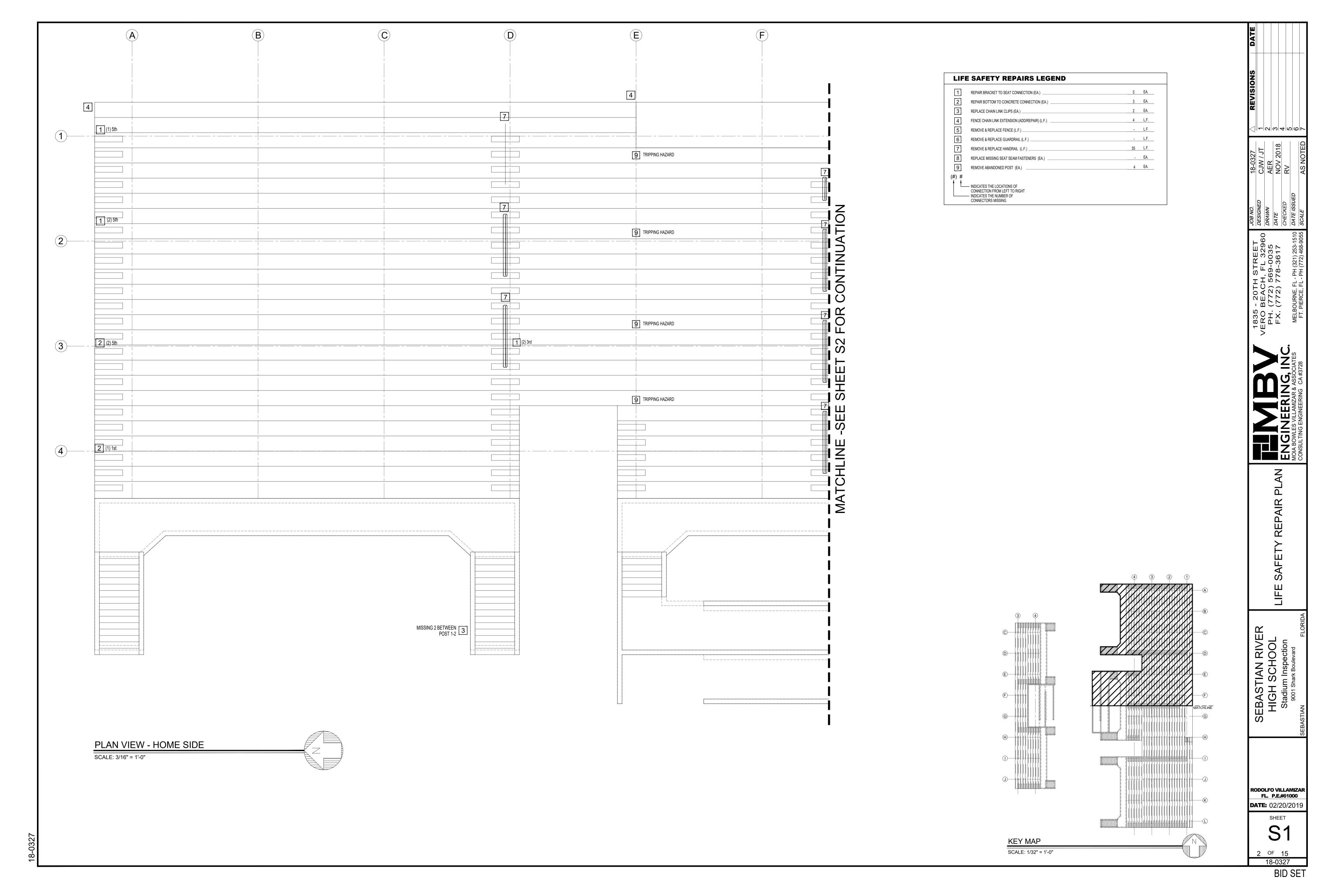
SCOPE OF WORK

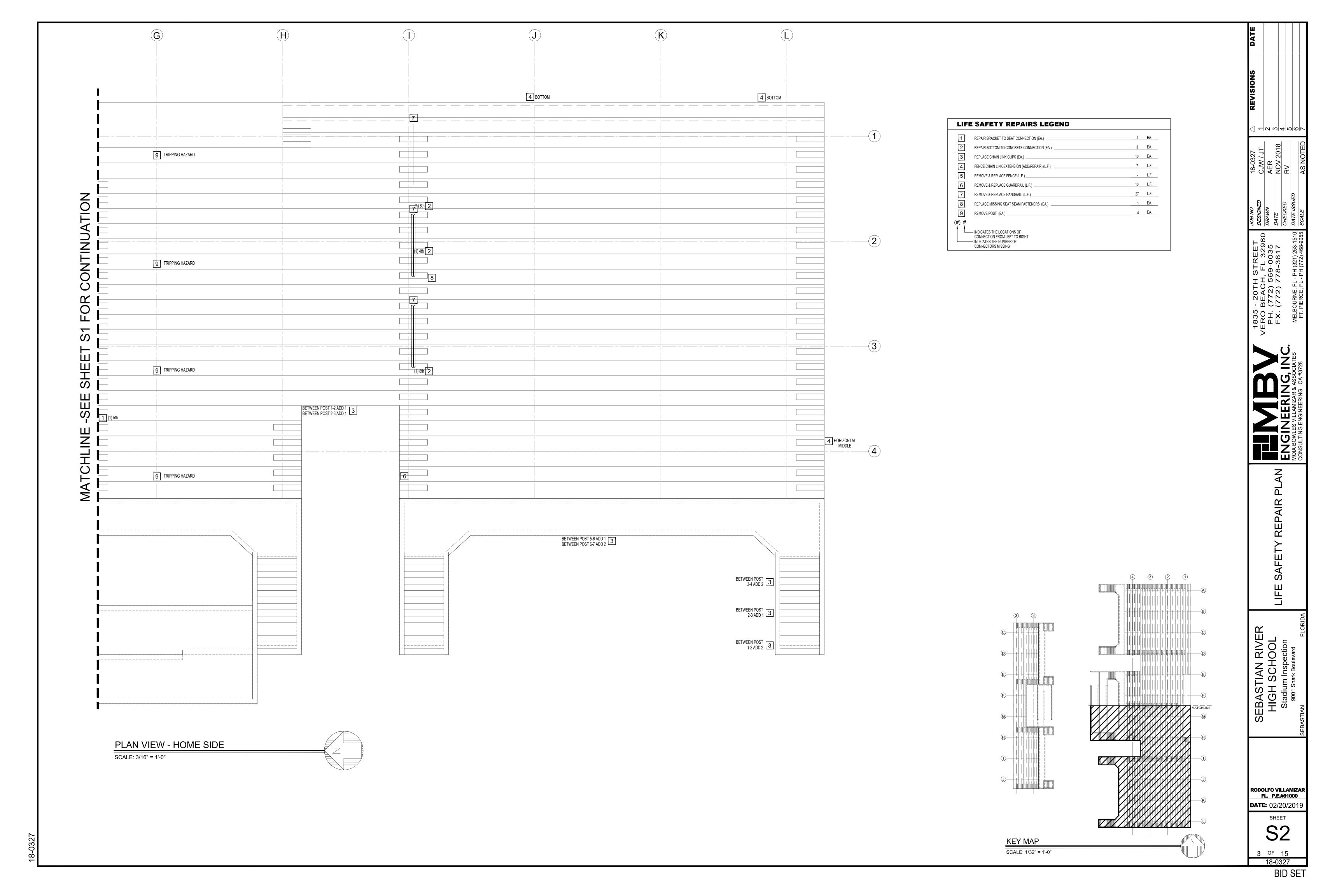
- REMOVE UNSOUND STUCCO AND CONCRETE AT DAMAGED AREAS PER DETAILS.
- REMOVE AND REPLACE HANDRAILS, GUARDRAILS, CHAIN-LINK FENCE PER PLANS.
- REMOVE AND REPLACE DIAGONAL X-BRACING PER PLANS. 4. FOLLOW THE INTERNATIONAL CONCRETE RESTORATION INSTITUTE'S RECOMMENDATIONS
- AND DETAILS ON THE PLANS 5. ALL FINISHES TO MATCH EXISTING.

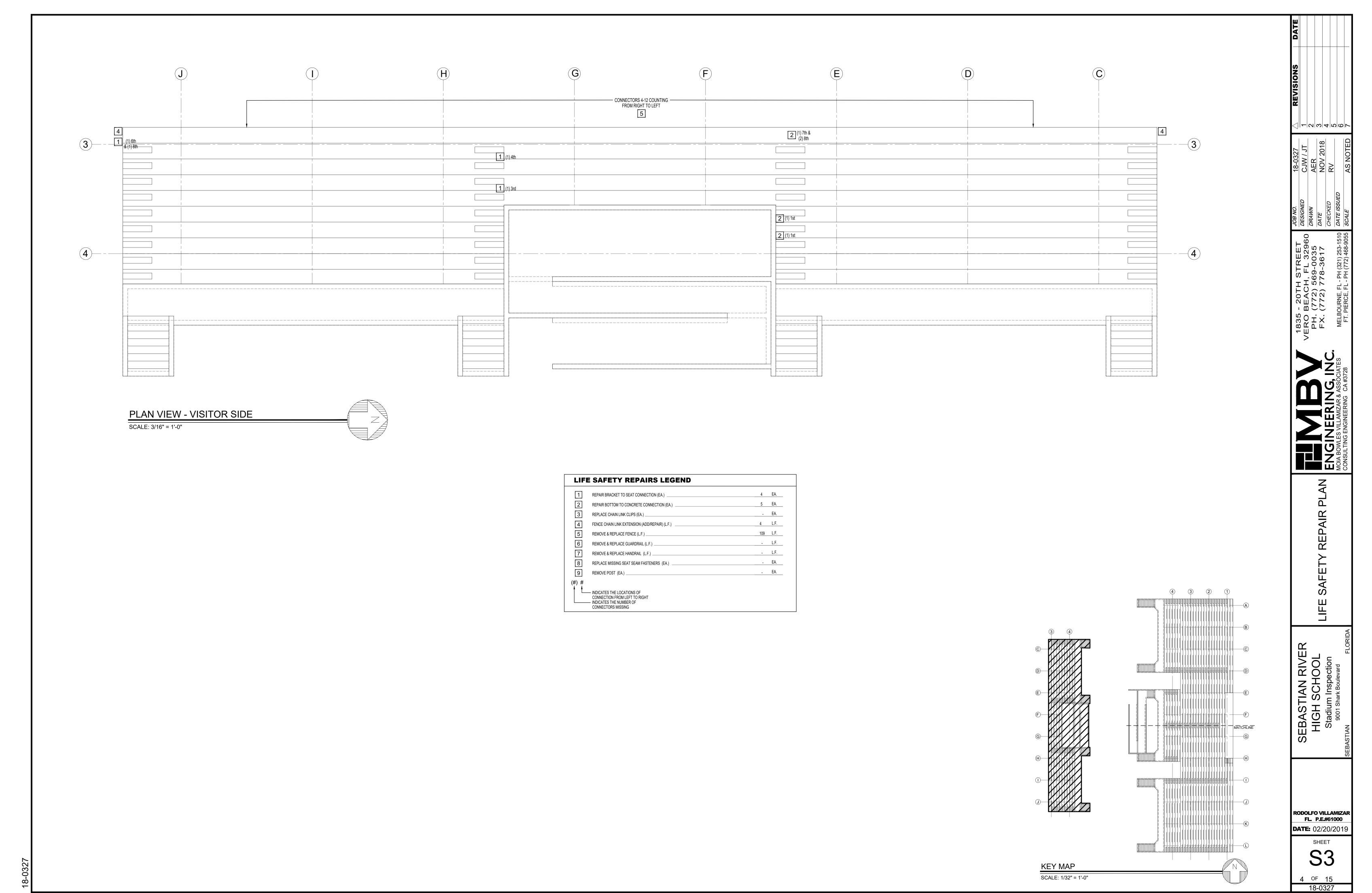
PER ORIGINAL CONSTRUCTION NOTES:

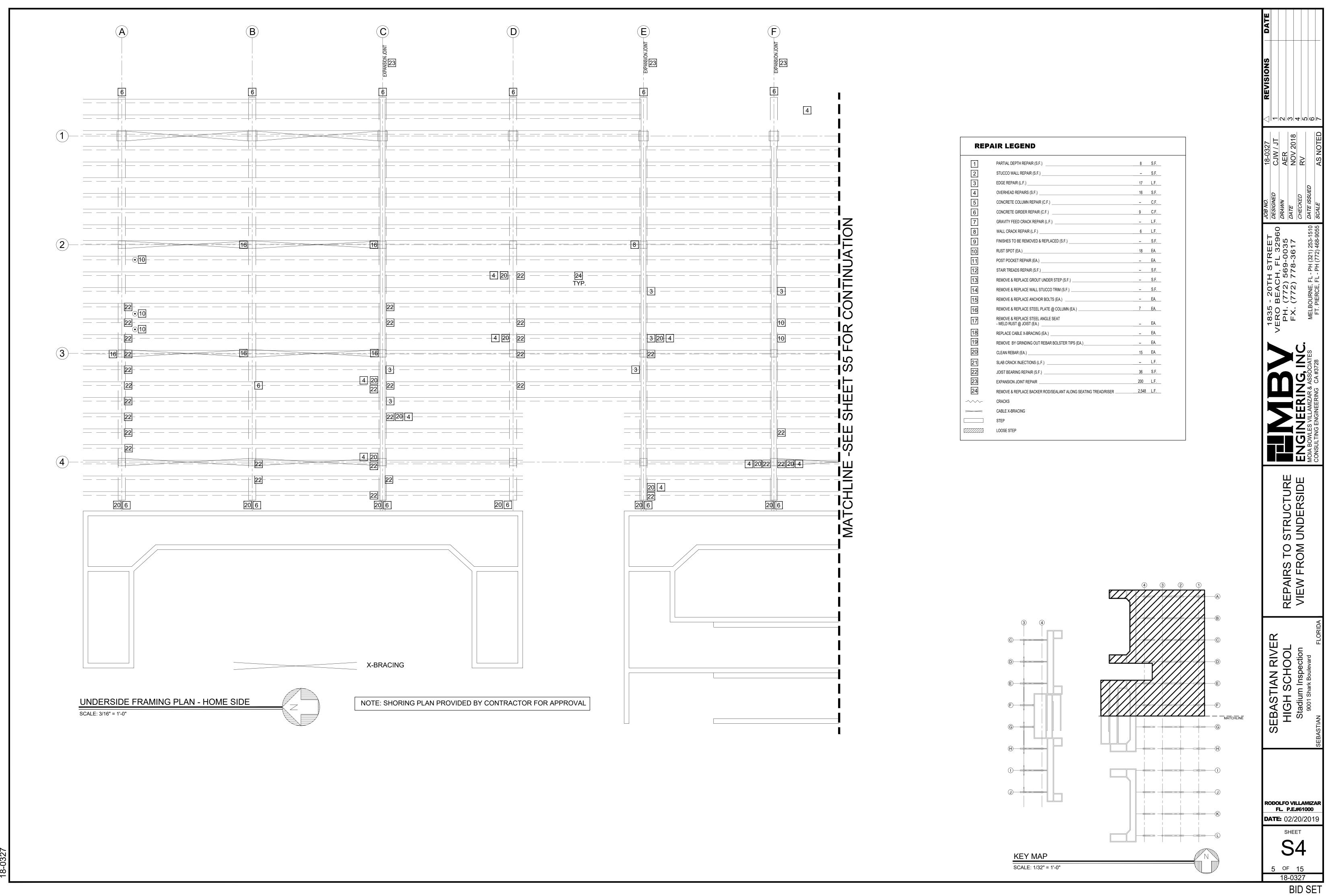
ALL EXPOSED STRUCTURAL STEEL AND CONNECTIVE HARDWARE SHALL BE HOT-DIP GALVANIZED AND TOUCHED UP AFTER WELDING. SHEET

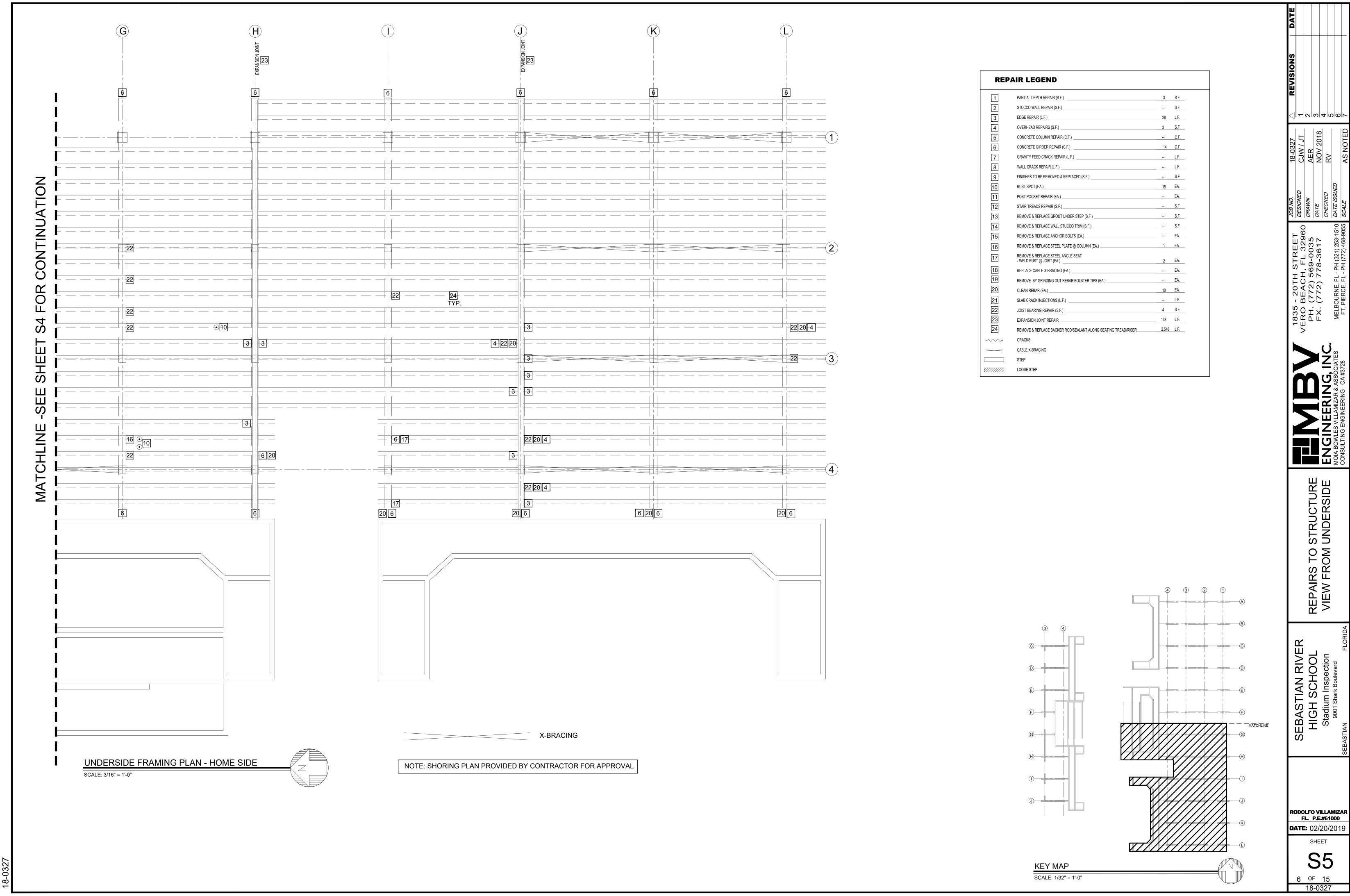
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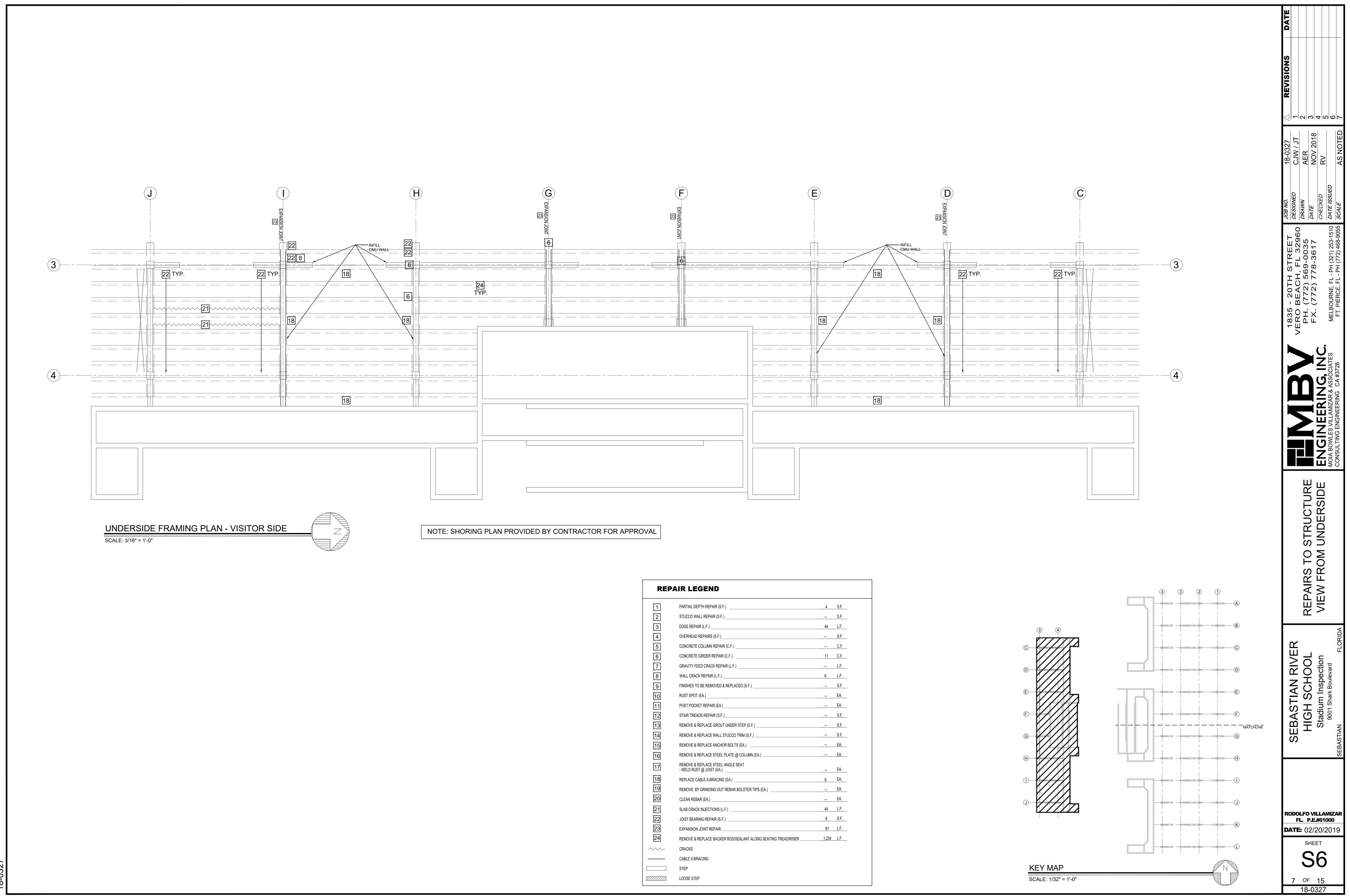


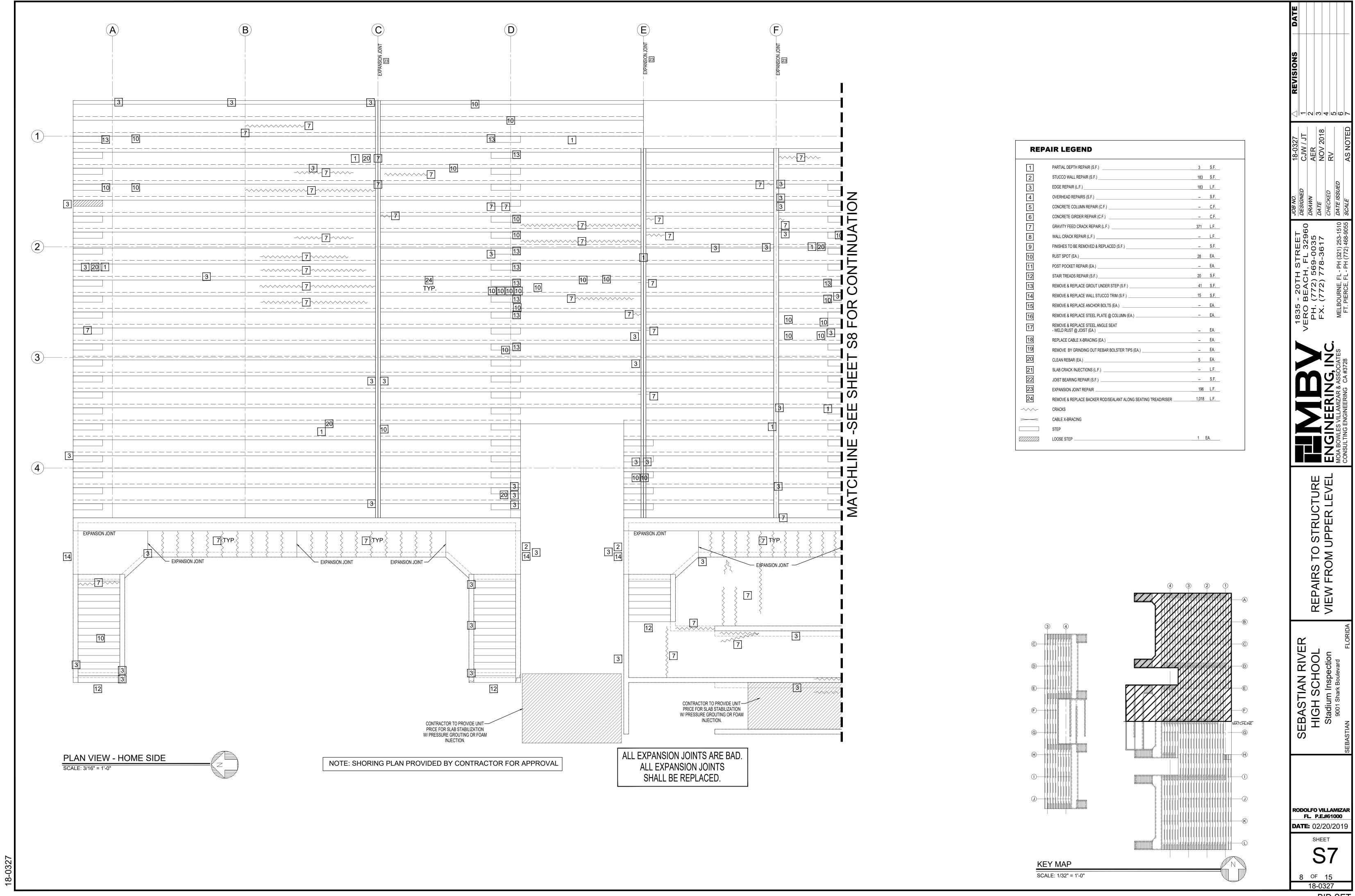


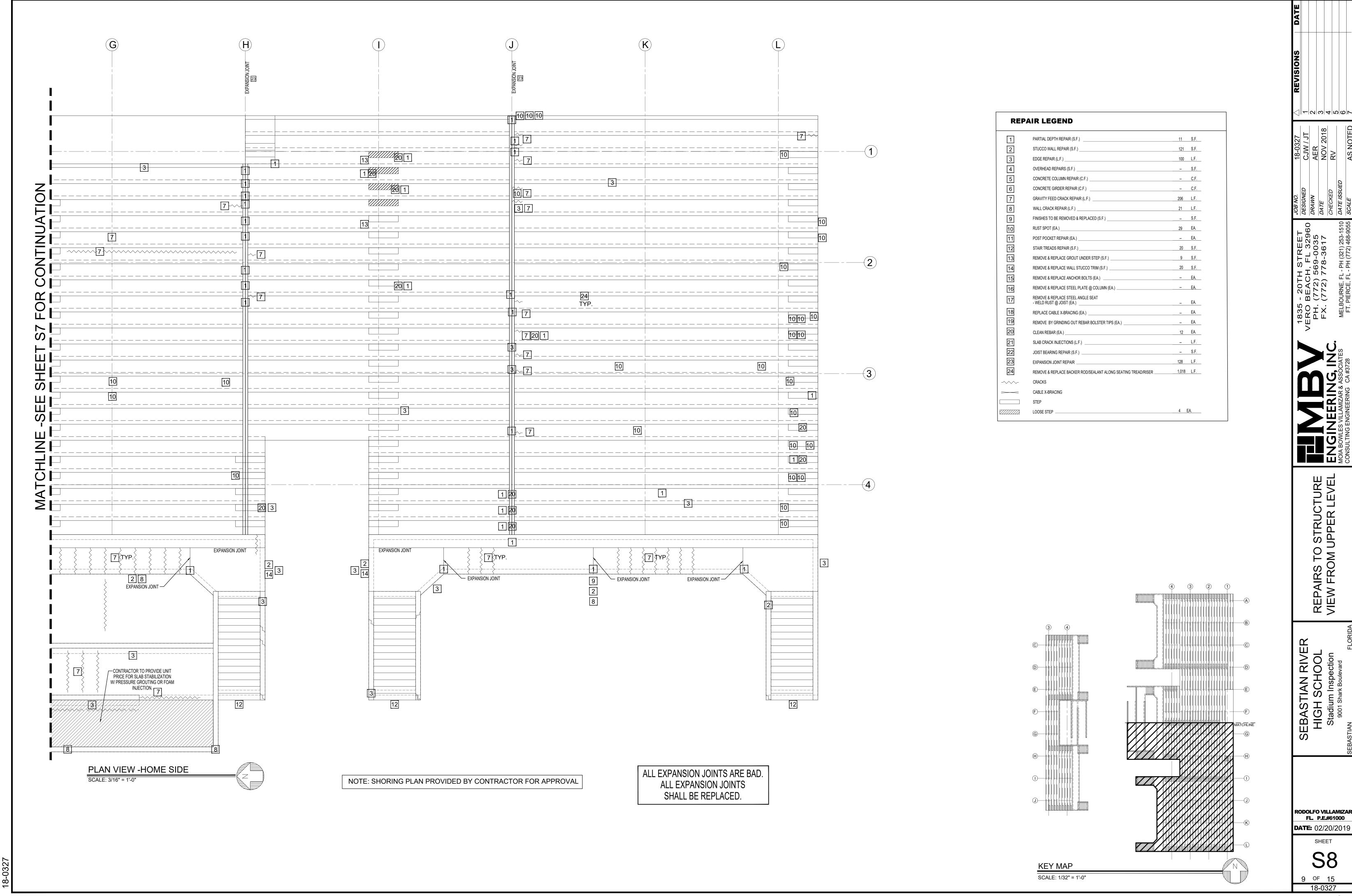


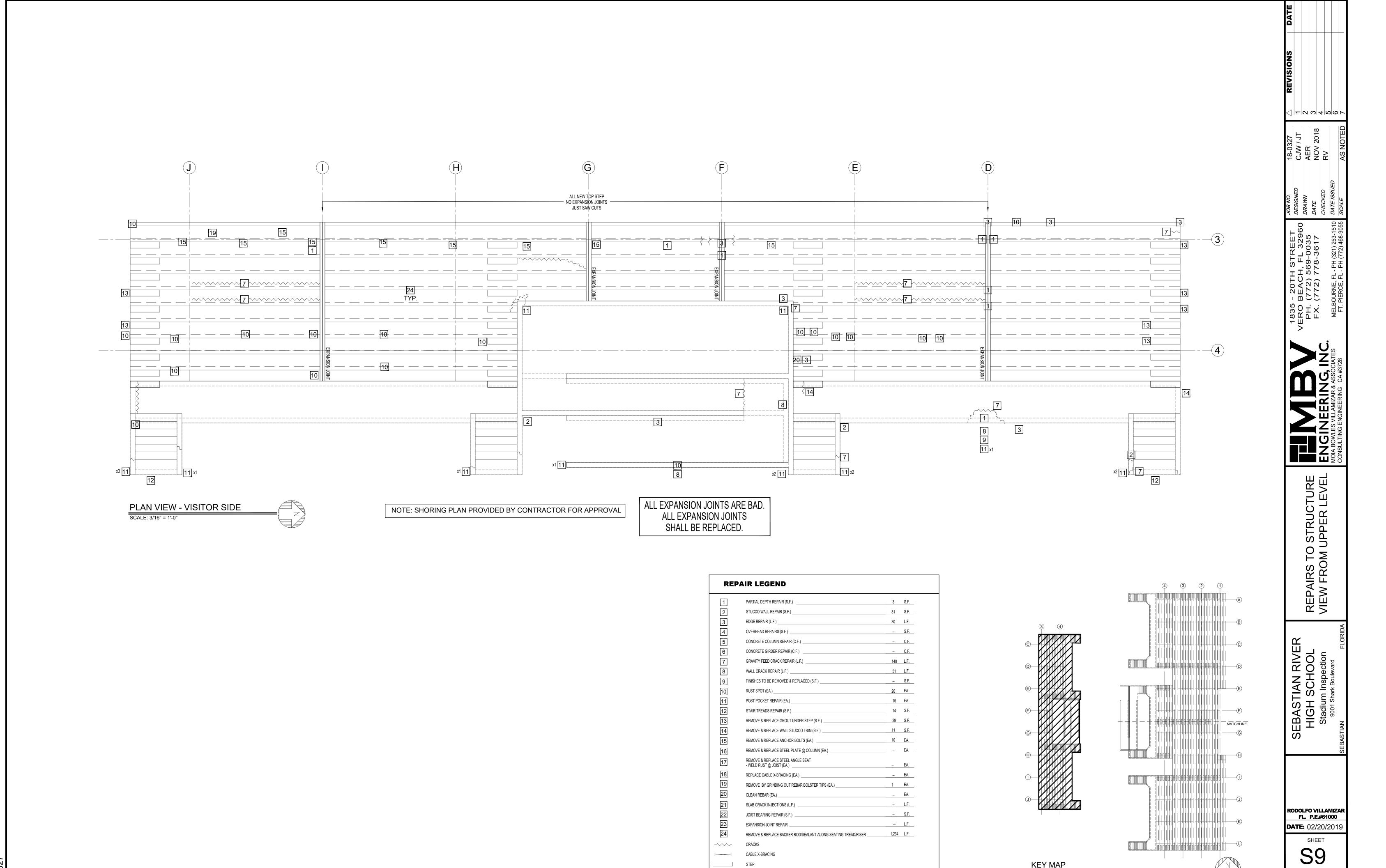










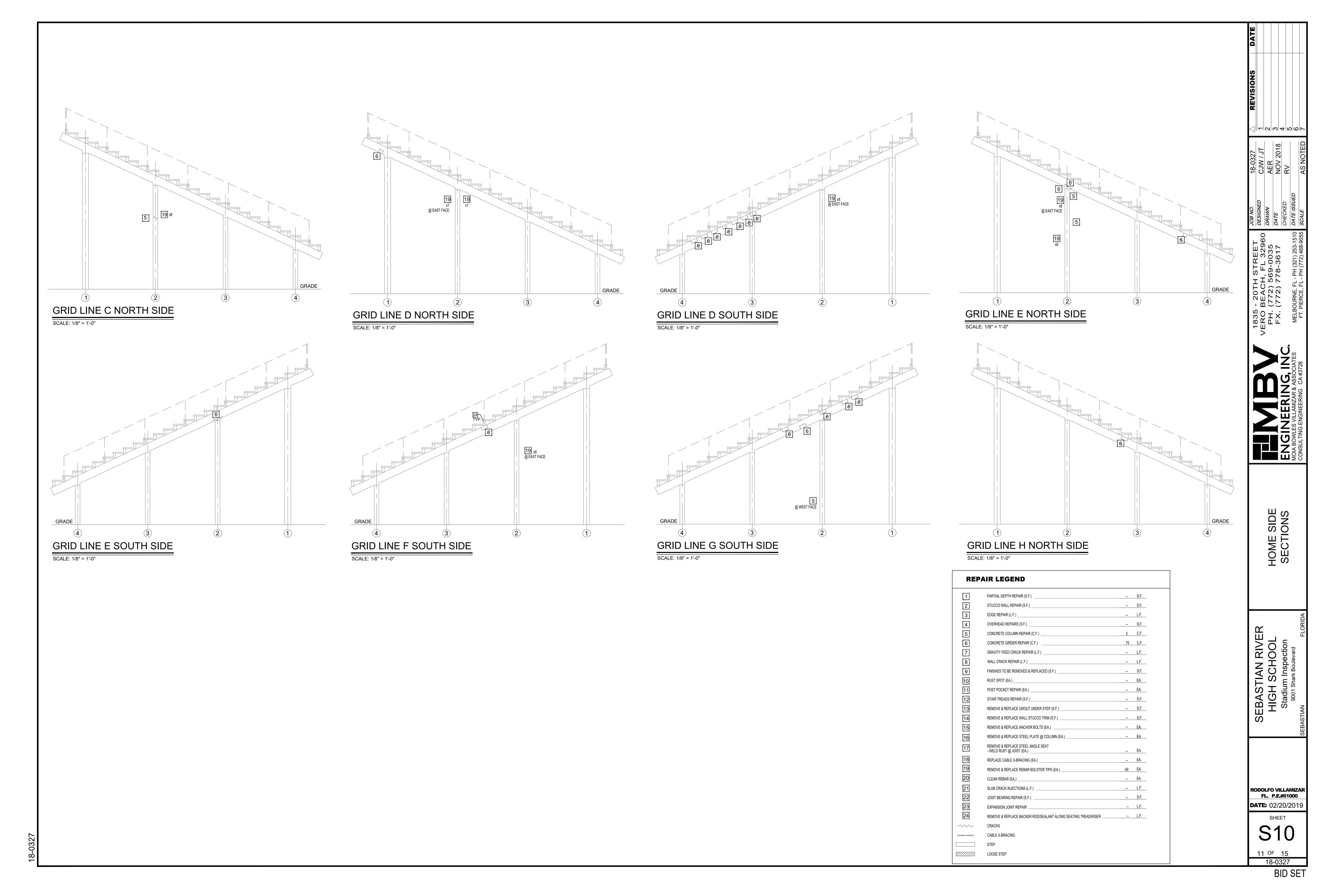


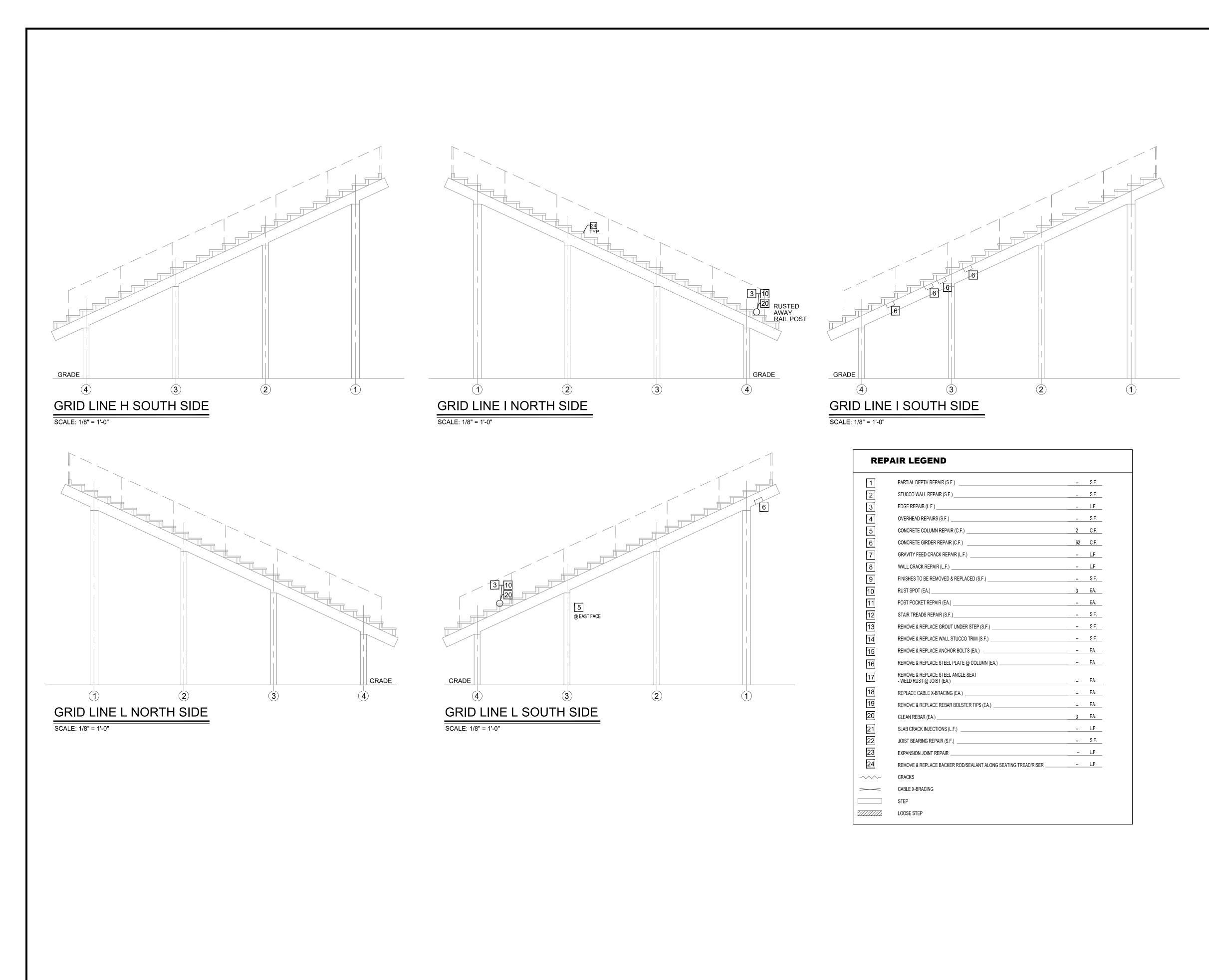
LOOSE STEP

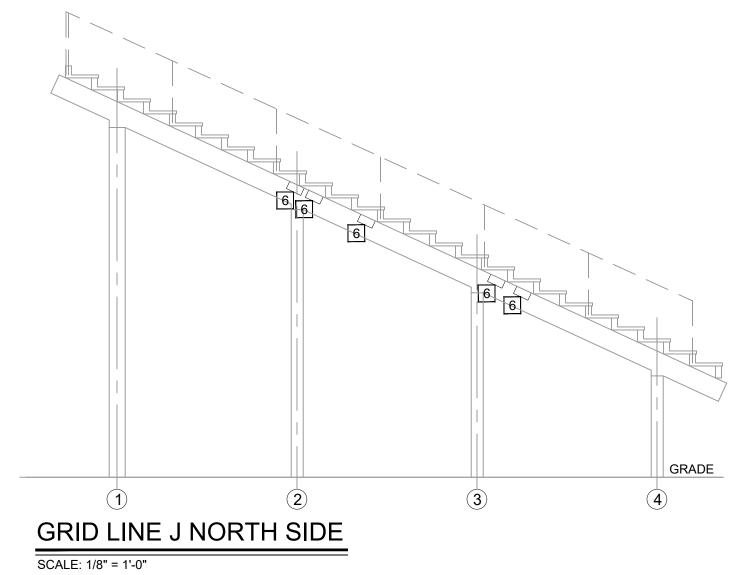
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SCALE: 1/32" = 1'-0"







ENGINEERING, INC.
MOIA BOWLES VILLAMIZAR & ASSOCIATES

HOME SIDE SECTIONS

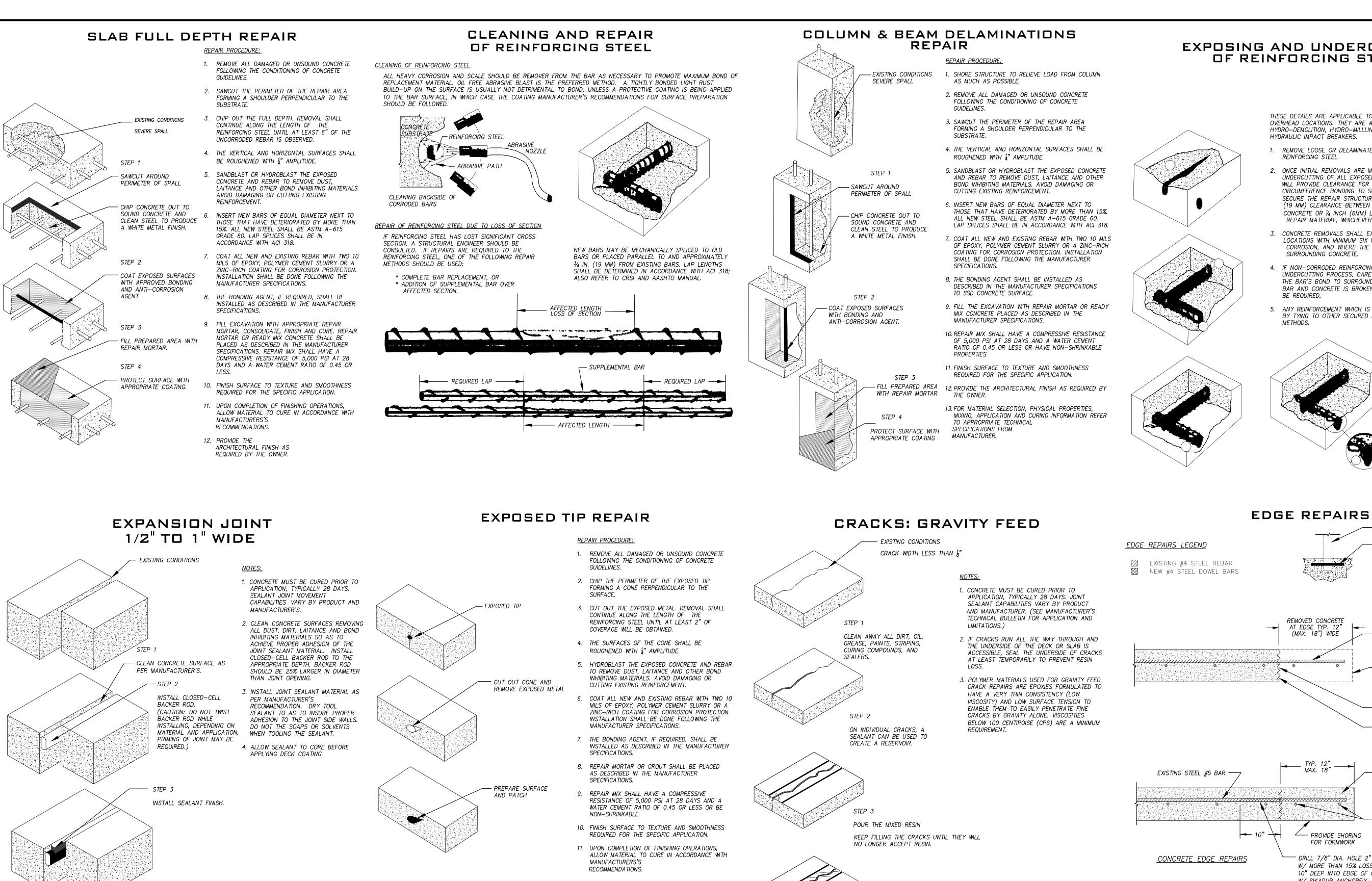
SEBASTIAN RIVER
HIGH SCHOOL
Stadium Inspection
9001 Shark Boulevard

RODOLFO VILLAMIZAR
FL P.E.#61000

DATE: 02/20/2019

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EXPOSING AND UNDERCUTTING OF REINFORCING STEEL

THESE DETAILS ARE APPLICABLE TO HORIZONTAL, VERTICAL AND OVERHEAD LOCATIONS. THEY ARE ALSO APPLICABLE TO REMOVAL BY HYDRO-DEMOLITION, HYDRO-MILLLING, AND ELECTRIC, PNEUMATIC OR

REMOVE LOOSE OR DELAMINATED CONCRETE ABOVE CORRODED

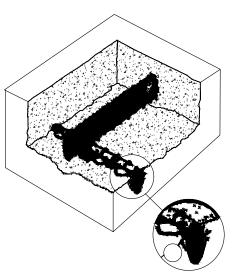
ONCE INITIAL REMOVALS ARE MADE, PROCEED WITH THE UNDERCUTTING OF ALL EXPOSED CORRODED BARS. UNDERCUTTING WILL PROVIDE CLEARANCE FOR UNDER BAR CLEANING AND FULL BAR CIRCUMFERENCE BONDING TO SURROUNDING CONCRETE, AND WILL SECURE THE REPAIR STRUCTURALLY. PROVIDE MINIMUM 3/4 INCH (19 MM) CLEARANCE BETWEEN EXPOSED REBARS AND SURROUNDING CONCRETE OR 1/4 INCH (6MM) LARGER THAN LARGEST AGGREGATE IN REPAIR MATERIAL, WHÌCHEVER IS GREATER.

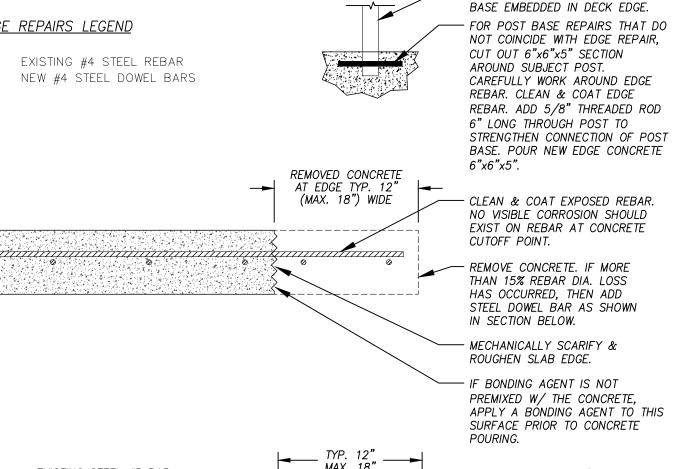
3. CONCRETE REMOVALS SHALL EXTEND ALONG THE BARS TO LOCATIONS WITH MINIMUM SIX INCHES FREE OF BOND INHIBITING CORROSION, AND WHERE THE BAR IS WELL BONDED TO

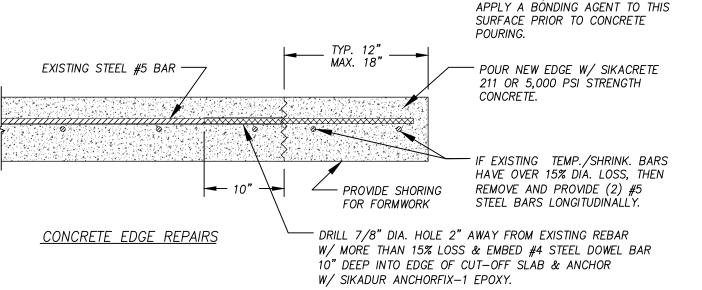
4. IF NON-CORRODED REINFORCING STEEL IS EXPOSED DURING THE UNDERCUTTING PROCESS, CARE SHALL BE TAKEN NOT TO DAMAGE THE BAR'S BOND TO SURROUNDING CONCRETE. IF BOND BETWEEN BAR AND CONCRETE IS BROKEN, UNDERCUTTING OF THE BAR SHALL

ANY REINFORCEMENT WHICH IS LOOSE SHALL BE SECURED IN PLACE BY TYING TO OTHER SECURED BARS OR BY OTHER APPROVED

2"x2" MIN. ALUMINUM RAIL POST







1. FINAL FINISH OF SLABS SHALL BE SMOOTH SPONGED SANDY FINISH OR AS SPECIFIED ON-SITE PER OWNER'S REQUIREMENTS.

ALLOW TIME FOR PENETRATION (1 HR MIN.)

AND REMOVE EXCESS.

2. FOR ANY TENSION BAR, MAXIMUM CONCRETE COVER SHALL BE 2" & MINIMUM COVER 1.5". TEMPERATURE/SHRINKAGE STEEL TO BE PLACED DIRECTLY UNDER TENSION STEEL.

3. ALL PRODUCT SUBSTITUTIONS MUST FIRST BE APPROVED BY THE ENGINEER.

4. ENGINEER SHALL BE IMMEDIATELY NOTIFIES CONCERNING ANY ENCOUNTERED DISCREPANCIES BETWEEN ACTUAL CONDITIONS & THIS DESIGN.

> RODOLFO VILLAMIZAF FL P.E.#61000 **DATE:** 02/20/2019

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CRACKS REPAIR: EPOXY INJECTION STRUCTURAL/WEARING SLAB

1. CONCRETE MUST BE CURED PRIOR TO APPLICATION, TYPICALLY 28 DAYS. EPOXY

CAPABILITIES VARY BY PRODUCT AND

LIMITATIONS.)

BY GRINDING.

EPOXY CAP SEAL.

MANUFACTURER. (SEE MANUFACTURER'S

TECHNICAL BULLETIN FOR APPLICATION AND

2. MECHANICALLY CLEAN SURFACE TO REMOVE

ALL DUST, DIRT, LAITANCE AND BOND

INHIBITING MATERIAL SO AS TO ACHIEVE

3. WHERE CONCRETE SURFACES ADJACENT TO CRACK ARE DETERIORATED, GROOVE THE

4. INJECT MIXED EPOXY RESIN INTO THE CRACK. INJECTION EQUIPMENT MUST HAVE SUFFICIENT PRESSURE TO FORCE RESIN INTO THE CRACK FOR FULL DISPLACEMENT.

5. FOR CRACK WIDTHS OF 0.010 IN. OR SMALLER

USE LOW VISCOSITY EPOXY (500 CPS OR

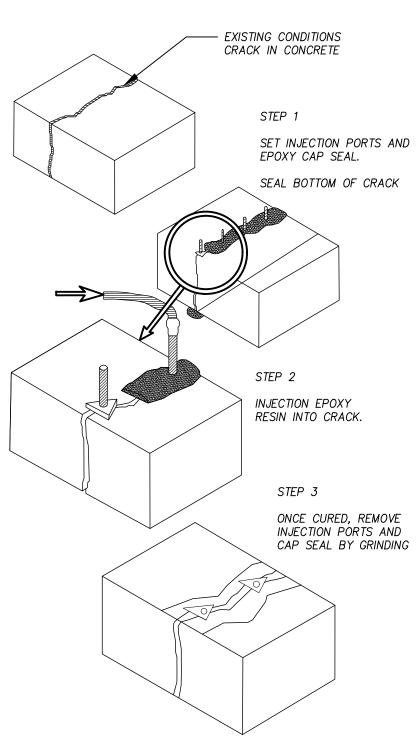
6. ALLOW THE EPOXY TO CURE TO SUFFICIENT HARDNESS BEFORE REMOVING INJECTION

7. UPON COMPLETION PREPARE THE CONCRETE

PORTS AND CAP SEAL FROM THE SURFACE.

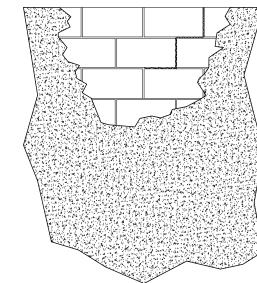
PROPER ADHESION OF THE INJECTION PORTS

CRACK UNTIL SOUND CONCRETE IS REACHED.



NOTES FOR GENERAL REPAIR: 1. CHIP AND REMOVE CRACKED MORTAR OR LOOSE

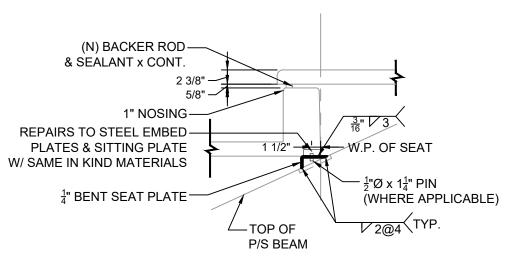
WALL CONCRETE.



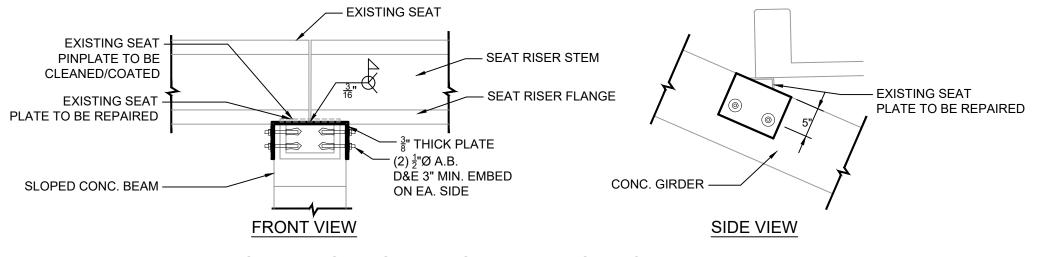
- 2. THOROUGHLY CLEAN MASONRY SUBSTRATE AND REMAINING STUCCO SURFACES (EXPOSED EDGES INCLUDED) WITH CONCRETE/MASONRY CLEANER OR TRISODIUM PHOSPHATE/WATER SOLUTION. 3. REPAIR CRACKED AND/OR SPALLED MASONRY SUBSTRATE INCLUDING CONCRETE BLOCK UNITS, MORTAR JOINTS, AND GROUT FILLED CELLS IN ACCORDANCE WITH REPAIR DETAILS AND

WALL REPAIR

- SPECIFICATIONS PROVIDED HEREIN. 4. REPAIR CRACKS WITH "UGL DRYLOK MASONRY CRACK FILLER", "ELASTIPOXY" OR SIMILAR EPOXY JOINT & CRACK FILLER MATERIAL; FOLLOWING THE MANUFACTURER'S RECOMMENDATIONS. FOR LARGER CRACKS TO PREVENT THE EPOXY FROM RUNNING OUT OF THE CRACK, INSERT FOAM BACKER
- ROD OF SUITABLE DIAMETER OR FILL IN WITH CAULK. FILL HOLLOW CORE HOLES WITH EXPANDABLE FOAM FILLER MATERIAL TO PROVIDE A BACKING SUBSTRATE. 6. COAT ALL SIDES OF GAP/HOLE WITH BONDING AGENT. 7. PATCH REPAIR AREAS WITH NEW REPAIR MORTAR SYSTEM OR EXTERIOR STUCCO SYSTEM IN ACCORDANCE WITH THE MANUFACTURER'S
- SPECIFICATIONS. 8. MATCH EXISTING FINISH TEXTURE AND COLOR AS
- CLOSE AS POSSIBLE. 9. FINISH/PAINT/COAT REPAIRED AREA IN ACCORDANCE WITH THE GENERAL NOTES.



SEAT RISER SETTING DETAIL - OPTION 1 SCALE: 3/4" = 1'-0"



SEAT RISER SETTING DETAIL - OPTION 2 SCALE: 3/4" = 1'-0"

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DATE: 02/20/2019

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